### STRUCTURAL NOTES

## 1. GENERAL

- A. THE BRIDGE MODIFICATIONS ARE DESIGNED UNDER THE PROVISIONS OF THE 2021 VIRGINIA STATEWIDE BUILDING CODE, 2021 INTERNATIONAL BUILDING CODE, ASCE 7-22, AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 8<sup>TO</sup> EDITION, AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN
- B. THE FOLLOWING LOADS WERE USED IN THE DESIGN:

LIVE LOAD (AASHTO) 90 PSF PEDESTRIAN BRIDGE DECK 4k WHEEL LOAD OVERLOAD CASE (H5 DESIGN VEHICLE)

SNOW LOAD GROUND SNOW LOAD (Pg) 25 PSF SNOW EXPOSURE FACTOR (Ce) 1.0 1.0

SNOW LOAD IMPORTANCE FACTOR (Is) THERMAL FACTOR (Ct)

PROJECT SCOPE DOES NOT MODIFY THE EXISTING LATERAL FORCE RESISTING SYSTEM OF THE BRIDGE. WIND AND SEISMIC LOADS ARE SHOWN FOR REFERENCE ONLY.

WIND LOAD (AASHTO) WIND SPEED (3-SECOND GUST) 115 MPH Vasd: (0.77\*VULT) 88.6 MPH WIND EXPOSURE 1.15 IMPORTANCE FACTOR (Ir) EARTHQUAKE DESIGN (AASHTO) SITE CLASSIFICATION SEISMIC RESPONSE COEFFICIENTS 0.16 Sd1 0.11 SEISMIC ZONE

HANDRAIL AND GUARD LOADS HANDRAIL AND GUARD 50 PLF / 200 LB PT LOAD 50 LB PT LOAD INTERMEDIATE RAIL

C. PROVIDE SUBMITTALS FOR THE FOLLOWING ITEMS:

--WOODEN STRUCTURAL MEMBER PRODUCT DATA

--WOOD POLYMER COMPOSITE DECKING PRODUCT DATA --CHAIN LINK FENCING PRODUCT DATA

--GALVANIZED CONNECTION HARDWARE INCLUDING BOLTS, NUTS, WASHERS, STAPLES, SCREWS ECT

### 2. GENERAL

- A. REVIEW ALL DRAWINGS AND SPECIFICATIONS BEFORE BEGINNING ANY WORK. NOTIFY EOR OF ANY DISCREPANCIES IN THE DRAWINGS, SPECIFICATIONS, OR ANY EXISTING CONDITIONS. ALL TRADES MUST REVIEW ALL DRAWINGS FOR WORK AFFECTING THEIR TRADE.
- B. CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE COMPLETE EXTENT AND QUANTITIES OF REPAIRS AND REPLACEMENT INDICATED IN THE DRAWINGS. THE EXTENT OF REQUIRED REPAIRS SHOWN ON THE CONSTRUCTION DRAWINGS ARE APPROXIMATE. FOR BID PREPARATION, CONTRACTOR SHOULD ARRANGE TO VISIT THE SITE FOR VERIFYING EXTENT OF REPAIRS AND REPLACEMENT, ASSESSING AVAILABLE STAGING AREAS, ASSESSING AVAILABILITY OF WATER AND ELECTRIC SERVICES, AND ASSESSING ACCESS TO THE WORK AREA.
- C. BY MEANS OF THE CONSTRUCTION DRAWINGS AND FIELD OBSERVATIONS, CONTRACTOR SHALL ESTABLISH AND SUBMIT BASE QUANTITIES AND UNIT PRICES FOR THE SCOPE OF WORK INCLUDED IN THE CONTRACT DOCUMENTS.
- D. THROUGH THE COURSE OF THE CONSTRUCTION, CONTRACTOR SHALL MAINTAIN AN ACCURATE RECORD OF AND REPORT REPAIR QUANTITIES IMPLEMENTED TO THE CONTRACTOR OFFICER/OWNER/PROJEC MANAGER/ENGINEER.
- E. CONTRACTOR IS RESPONSIBLE FOR SCHEDULING THE SEQUENCE OF CONSTRUCTION WORK AND COORDINATION THE ACTIVITIES OF ALL TRADES.
- F. CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO PROCEEDING WITH THE WORK. DISCREPANCIES IN ASSUMED FRAMING DIRECTION OR SIZES AND ANY INDICATION OF STRUCTURAL DEFICIENCY NOTED DURING THE PERFORMANCE OF THE WORK SHALL BE CONVEYED IN WRITING TO THE CONTRACTOR OFFICER / OWNER / PROJECT MANAGER / ENGINEER.
- G. CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING AND SHORING REQUIRED TO EXECUTE THE WORK AND PROTECT PERSONS AND PROPERTY PRIOR TO AND DURING CONSTRUCTION. THE CONTRACTOR SHALL ADJUST AND MAINTAIN TEMPORARY BRACING AND SHORING AS REQUIRED THROUGHOUT
- H. STRUCTURAL ELEMENTS TO REMAIN SHALL NOT BE CUT IN A MANNER RESULTING IN THE REDUCTION OF LOAD-CARRY CAPACITY. DO NOT ADD LOADS TO ANY POINT UP UPON THE EXISTING STRUCTURE WHICH COULD OVERSTRESS THE STRUCTURE.
- I. COLLECT, TRANSPORT AND LEGALLY DISPOSE OF ALL CONSTRUCTION TRASH AND DEBRIS AT LOCATION OFF THE PROPERTY. SITE SHALL BE CLEAN AND NEAT.
- J. ALL REPAIRS SHALL BE PER STRUCTURAL DETAILS AND SPECIFICATIONS.

# 3. DEMOLITION

- A. THE CONTRACTOR IS FULLY RESPONSIBLE FOR THE MEANS AND METHODS OF DEMOLITION AND THE INTEGRITY AND STABILITY OF THE EXISTING STRUCTURE UNTIL THE WORK IS COMPLETED. PROVIDE ADEQUATE SHORING, BRACING AND OTHER TEMPORARY SUPPORT DURING DEMOLITION.
- B. CONTRACTOR SHALL VERIFY THAT EXISTING CONSTRUCTION CORRESPONDS TO THAT SHOWN ON THE DRAWINGS. DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER.
- C. UNTIL PROPERLY SHORED, DO NOT CUT EXISTING STRUCTURAL MEMBERS IN A MANNER RESULTING IN A REDUCTION OF LOAD-CARRYING CAPACITY. DO NOT EXCEED THE CAPACITY OF THE EXISTING STRUCTURE WITH SUPERIMPOSED LOADS, INCLUDING BUT NOT LIMITED TO CONSTRUCTION/DEMOLITION EQUIPMENT OR
- D. IN GENERAL, SELECTIVE STRUCTURAL DEMOLITION IS TO BE PERFORMED BY REMOVING EXISTING CONNECTION HARDWARE (SCREWS AND BOLTS) AND WITH PHYSICAL CUTTING ACTION (I.E. SAWING AND GRINDING INSTEAD OF HAMMERING AND CHOPPING).
- E. THE CONTRACTOR IS RESPONSIBLE, AT THEIR EXPENSE, TO REPAIR ANY DAMAGE THAT HAS OCCURRED TO STRUCTURAL ELEMENTS OR FINISH MATERIALS DURING THE DEMOLITION PROCESS.
- 4. WOOD-POLYMER COMPOSITE DECKING (WPC)
- A. WOOD-POLYMER COMPOSITE (WPC) DECKING SHALL BE TREX TRANSCEND WPC LUMBER, AS MANUFACTURED BY TREX COMPANY, LLC, WINCHESTER, VA 22602, OR APPROVED EQUIVALENT. WPC LUMBER SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES: BENDING STRESS "Fb" = 500 PSI HORIZONTAL SHEAR "Fv" = 360 PSI COMPRESSION PERPENDICULAR TO GRAIN "Fc" = 540 PSI COMPRESSION PARALLEL TO GRAIN "Fc11" = 540 PSI MODULUS OF ELASTICITY "E" = 200,000 PSI

- B. WPC DECKING SHALL BE "1X6" NOMINAL OR 0.94"X5.5" ACTUAL AND SHALL BE FASTENED TO WOOD BLOCKING WITH TWO ROWS OF #8X2-1/2 INCH DECK SCREWS AT A MAXIMUM SPACING OF 16 INCHES, ON CENTER. DECKING SHALL BE INSTALLED ACROSS THE ENTIRE WIDTH OF THE BRIDGE DECK WITHOUT
- C. WPC DECKING ATTACHMENT SCREWS SHALL BE #8 x 2-1/2" LONG DECK SCREWS WITH GALVANIZED, EPOXY OR APPROVED ALTERNATE COATING.

# WOOD FRAMING

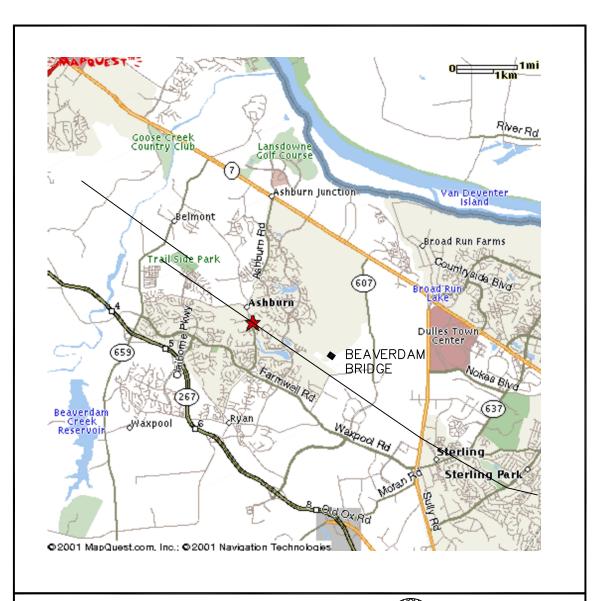
- A. ALL EXTERIOR FRAMING SHALL BE PRESSURE-TREATED (PT). FRAMING SHALL BE PRESSURE-TREATED WITH ALKALINE COPPER QUAT (ACQ) OR COPPER AZOLE (CBA-A AND CA-B), NOT SODIUM BORATE (SBX). PT LUMBER SHALL NOT BE INCISED.
- B. STRUCTURAL POSTS AND PRESSURE-TREATED LUMBER (PT) SHALL BE SOUTHERN PINE (SP) #2 OR BETTER, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

BENDING STRESS "Fb" = 750 PSI SINGLE MEMBER USE HORIZONTAL SHEAR "Fv" = 175 PSI COMPRESSION PERPENDICULAR TO GRAIN "Fc" COMPRESSION PARALLEL TO GRAIN "Fc11" = 1,250 PSI MODULUS OF ELASTICITY "E" = 1,400,000 PSI

## C. BOLTS AND FASTENERS SHALL BE HOT-DIPPED GALVANIZED

- D. ALL BOLTED CONNECTIONS, INCLUDING THRU BOLTS AND LAGS SHALL HAVE A STANDARD CUT METAL WASHER BETWEEN THE WOOD MEMBER AND NUT, UNLESS A LARGER PLATE WASHER OR OTHER IS NOTED ON THE DRAWINGS.
- E. ALL WOOD SCREWS SHALL BE HOT DIPPED GALVANIZED OR OTHERWISE PROTECTED WITH AN APPROVED
- F. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS.
- 6. PVC COATED STEEL CHAIN LINK FENCE FABRIC
- A. FENCING SHALL BE ASTM F668 2" MESH, 9 GAUGE CORE WIRE WITH BLACK CLASS 2B COATING.
- B. STAPLES FOR FENCE ATTACHMENT SHALL BE 1-1/2" x 1/4" FABRICATED FROM 9 GAUGE HOT DIPPED GALVANIZED SMOOTH SHANK WIRE.

WARNING: THE STRUCTURAL INTEGRITY OF THE BRIDGE SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF-BRACING UNTIL PERMANENTLY AFFIXED TO THE STRUCTURE. THE STRUCTURAL ENGINEERS ASSUME NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.



SITE LOCATION

#### ANCHOR BOLT KIP(S) ABV KNOCK-OUT ABOVE ADDL ADDITIONAL KIPS PER SQ. INCH ADJ ADJACENT AFF ABOVE FINISH FLOOR LINTEL MARK/STEEL ANGLE ALTERNATE APPROX APPROXIMATE(LY) LONG LEG HORIZONTAL ARCH ARCHITECT(URAL) LLV LONG LEG VERTICAL LIVE LOAD LOW POINT BEAM MARK, SOIL BORING MARK LONG SIDE HORIZONTAL BOTTOM OF FOOTING ELEVATION LSV LONG SIDE VERTICAL BRACED FRAME MARK LAMINATED VENEER LUMBER/LEVEL LVL BLOCKING BLDG BUILDING BLW BELOW MANUF MANUFACTURER(ED) вм MAS MASONRY BOD **BOTTOM OF DECK** MAXIMUM BOS **BOTTOM OF STEEL** MECH MECHANICAL BOTT MECHANICAL, ELECTRICAL, PLUMBING BP **BEARING PLATE MARK** MINIMUM BRG BEARING MISC MISCELLANEOUS BSMT BASEMENT MASONRY OPENING BTWN BETWEEN MATL MATERIAL MTL METAL COLUMN MARK CIP CAST IN PLACE NTS NOT TO SCALE CONTROL/CONSTRUCTION JOINT NEAR SIDE COMPLETE JOINT PENETRATION WELD NIC CJP NOT IN CONTRACT CL CENTER LINE/COLUMN LINE CLR CLEAR(ANCE) CMU CONCRETE MASONRY UNIT ON CENTER(S) COL COLUMN OPNG OPENING COM CENTER OF MASONRY WALL OPPOSITE OPP COMP COMPOSITE OUTSIDE FACE CONC CONCRETE CONN CONNECTION CONST CONSTRUCTION PIER MARK CONT CONTINUOUS POWDER ACTUATED FASTENER COORD COORDINATE(TION) PRECAST CONCRETE cos CENTER OF STUD PDF POWER DRIVEN FASTENER PEB PRE-ENGINEERED BUILDING PERIM PERIMETER DBA DEFORMED BAR ANCHORS PLATE DTL POUNDS PER LINEAR FOOT DIAMETER PLUMBING DIAG DIAGONAL PRECAST PLANK MARK DIM DIMENSION PROJECTION DN DOWN PSF POUNDS PER SQ. FOOT DWG DRAWING POUNDS PER SQ. INCH DBL DOUBLE PARALLEL STRAND LUMBER COLUMN DEAD LOAD POST TENSION(ED)/PRESSURE TREATED EACH END QUANTITY QTY FACH FACE ELEVATION ELECTRICAL RADIUS RAD ELEV **ELEVATOR** ROOF DRAIN RD EOD EDGE OF DECK REV REVISION, REVISE(D) EOJ EDGE OF JOIST REINF REINFORCE(D), (ING) EOS EDGE OF SLAB REM REMAINDER REQD REQUIRED EQUIP EQUIPMENT ROOF TOP UNIT RTU EACH SIDE EXISTING TO REMAIN ETR **EACH WAY** STRAP BEAM, SLAB BEAM EXIST, EX EXISTING **EXPANSION** SCHED SCHEDULE(D) **EXTERIOR** EXT SPECIALTY DESIGN ENGINEER SIMILAR SJI STEEL JOIST INSTITUTE FOOTING MARK SLAB ON GRADE SOG FLOOR DRAIN SQ SQUARE FOUNDATION FDN STD STANDARD FINISH STL STEEL FLR FLOOR STRUCT STRUCTURAL FOB FACE OF BUILDING SPA SPACES FOM FACE OF MASONRY WALL SNOW LOAD FOS FACE OF STUD STAINLESS STEEL FRT FIRE RETARDANT TREATED FOOTING STEP/FAR SIDE FTG FOOTING TEMPORARY FUT **FUTURE** TEMP TOP OF FOOTING ELEVATION TOP OF GRADE BEAM GAGE, GAUGE THICK(NESS), (ENED) GALVANIZED WOOD I JOIST GALV THROUGH OUT GRADE BEAM TOC TOP OF CONCRETE GENERAL CONTRACT(OR) GC TOP OF PIER ELEVATION GIRDER TRUSS TOS TOP OF STEEL ELEVATION TOP OF WALL ELEVATION TOW TYP TYPICAL HORIZ HORIZONTAL HIGH POINT HIGH STRENGTH UNEXC UNEXCAVATED HOLLOW STRUCTURAL SECTION HSS UNO UNLESS NOTED OTHERWISE HEIGHT UNDERSIDE METAL DECK ELEVATION UMD HTR HIP TRUSS VERTICAL VERIFY IN FIELD INFORMATION INFO INSIDE FACE WIND FRAME **WORK POINT** JBE JOIST BEARING ELEVATION WOOD-POLYMER COMPOSITE JST JOIST WELDED WIRE FABRIC JOINT JTR JACK TRUSS

ABBREVIATIONS LEGEND

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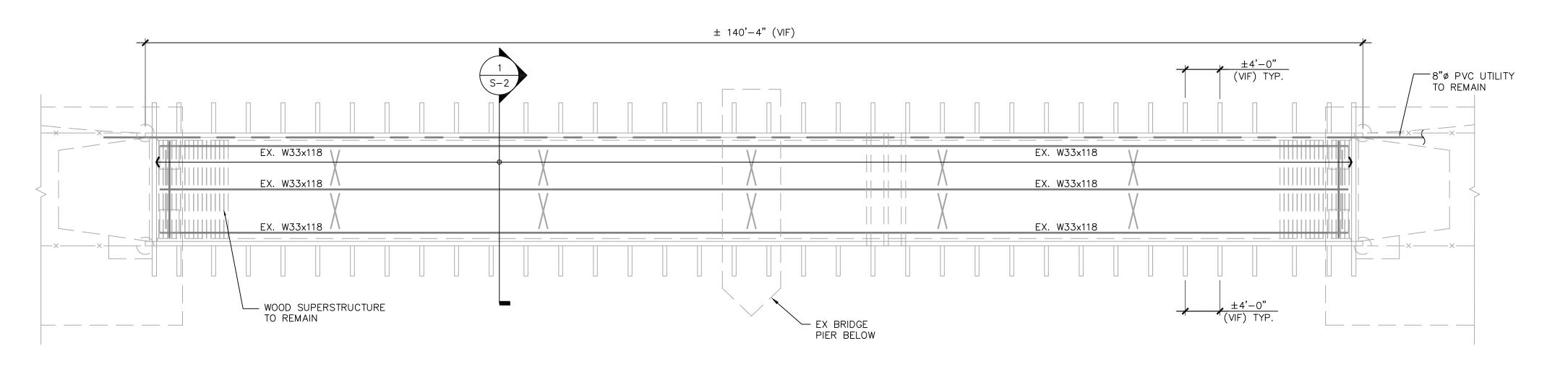
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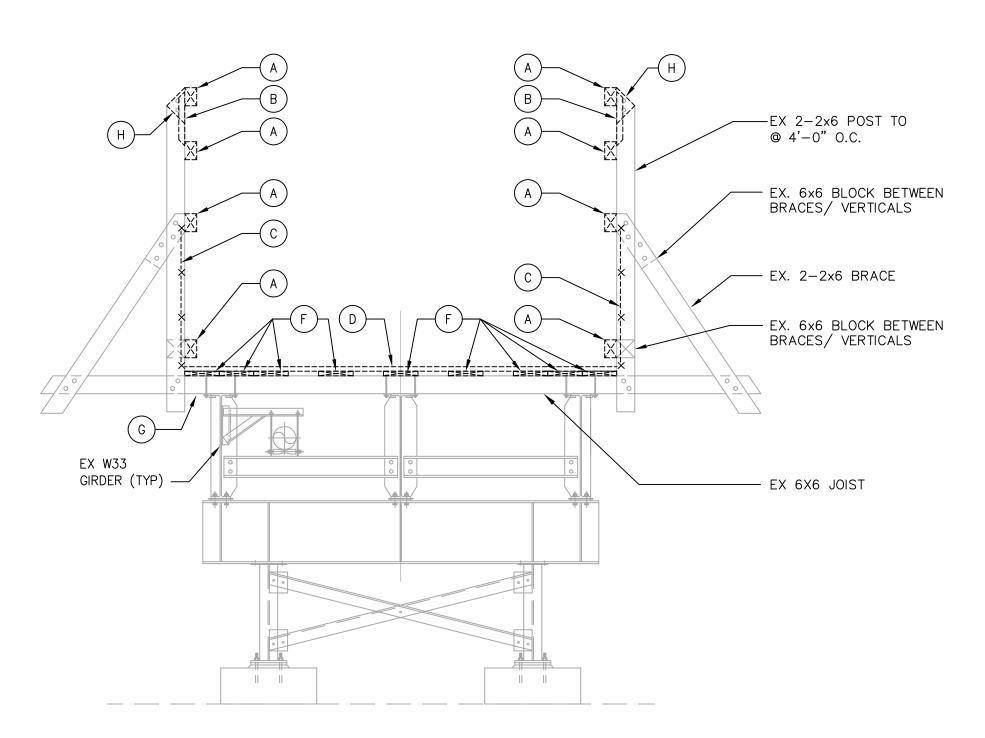


# DEMOLITION PLAN

SCALE: 1/8" = 1'-0"

NOTES:

 ALL DIMENSIONS SHOWN ARE APPROXIMATE AND SHALL BE VERIFIED IN THE FIELD.



# TYPICAL DEMOLITION SECTION (1)

SCALE: 3/8"=1'-0"

ALE. 3/0 =1-0

- 1. KEYED DEMOLITION NOTES DENOTE THE FOLLOWING AND APPLY TO FULL EXTENT OF BRIDGE:
- A DEMOLISH EXISTING 4x6 GUARDRAILS.
- B DEMOLISH EXISTING 2x2 GUARDRAIL PICKETS AT +/- 6" OC.
- C DEMOLISH EXISTING +/- 4'-0" TALL CHAIN LINK GUARDRAIL INFILL PANEL.
- D DEMOLISH EXISTING WOOD-POLYMER COMPOSITE DECKING (WCP).
- F DEMOLISH EXISTING 2x12 RUNNER BOARDS.
- DEMOLISH EXISTING 6X6 JOISTS WHERE ROT, DETERIORATION AND SECTION LOSS IS FOUND AND REPLACE IN KIND WITH 6x6 JOISTS SPANNING FULL WIDTH OF BRIDGE. VERIFY CONDITION OF EACH MEMBER IN THE FIELD AFTER WCP DECKING IS REMOVED. IDENTIFY WHERE REPAIRS ARE REQUIRED BY PERFORMING A VISUAL INSPECTION AND BY MANUALLY DRIVING A WOOD AWL INTO SUSPECTED DAMAGE TO IDENTIFY SOFT / COMPROMISED MATERIAL. IDENTIFY AND TAG DAMAGED 6x6 JOISTS IN THE FIELD AND CONTACT ENGINEER TO SCHEDULE A SITE VISIT TO REVIEW THE PLANNED REPLACEMENT LOCATIONS PRIOR TO REMOVING ANY EXISTING 6x6 JOISTS. FOR BID PURPOSES ASSUME THAT 50% OF THE EXISTING MEMBERS REQUIRE REPLACEMENT. INCLUDE IN BID AN ADD ALTERNATE UNIT PRICE PER JOIST FOR DEMOLITION AND REPLACEMENT OF 6x6 JOISTS BEYOND THE 50% THRESHOLD. PRESERVE EXISTING STEEL CLIPS THAT ANCHOR THE JOISTS TO THE EXISTING W33 GIRDERS, AND RE-INSTALL ON NEW 6X6 JOISTS WITH GALVANIZED 5/8" DIAMETER CARRIAGE BOLTS WITH NUT AND LOCK NUT. SEE 3/S-4.
- H DEMOLISH EXISTING 6x6 BLOCKING BETWEEN VERTICAL POSTS
- 2. REFER TO DRAWING S-3 FOR LOCATIONS OF SELECTIVE DEMOLITION AND REPAIRS.
- 3. INCLUDE REMOVAL AND DISPOSAL OF ALL DEMOLISHED MATERIAL IN BID.



DEMOLITION PLAN AND SECTION

NVRPA - BRIDGE DECKING IMPROVEMENT

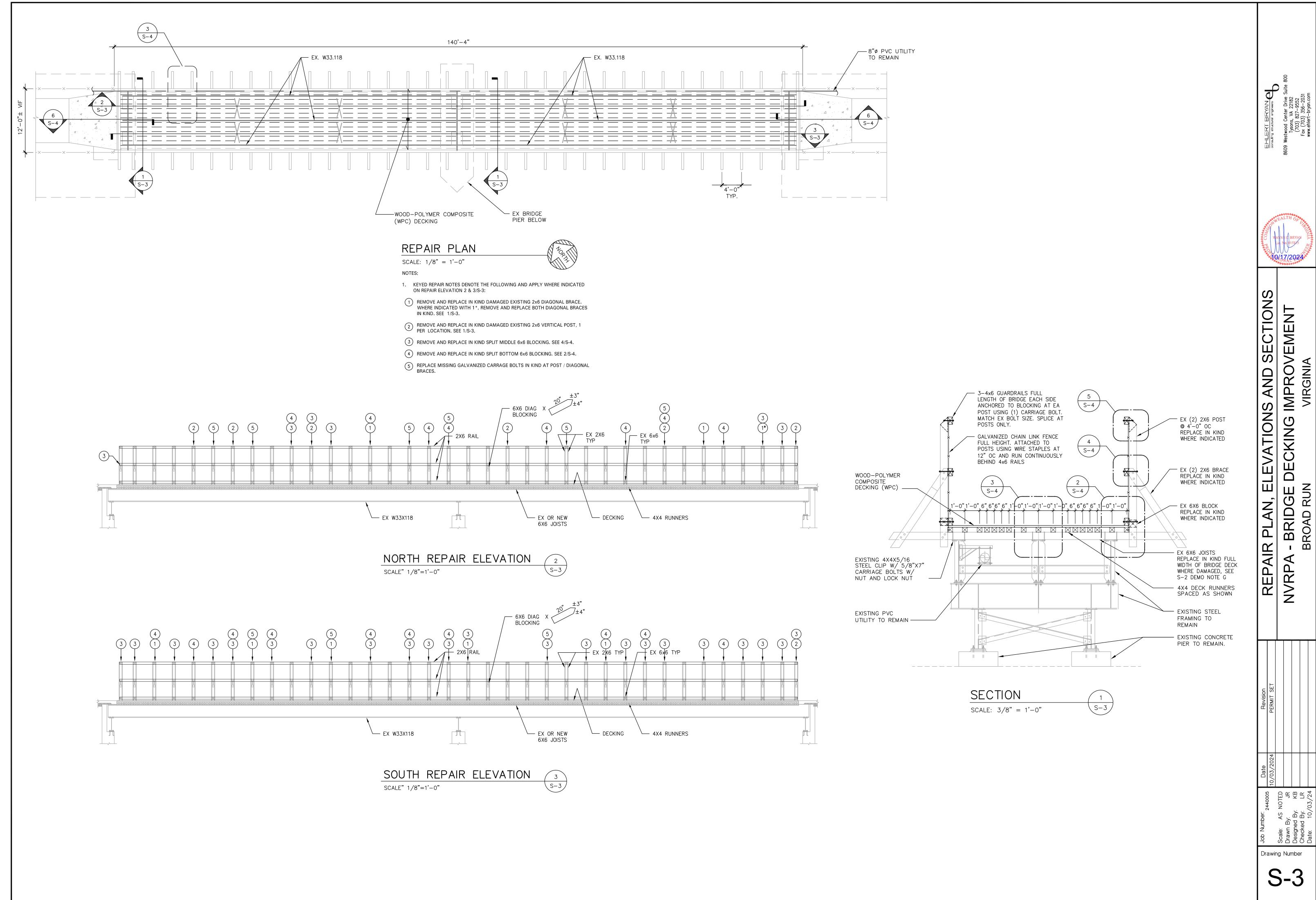
BROAD RUN

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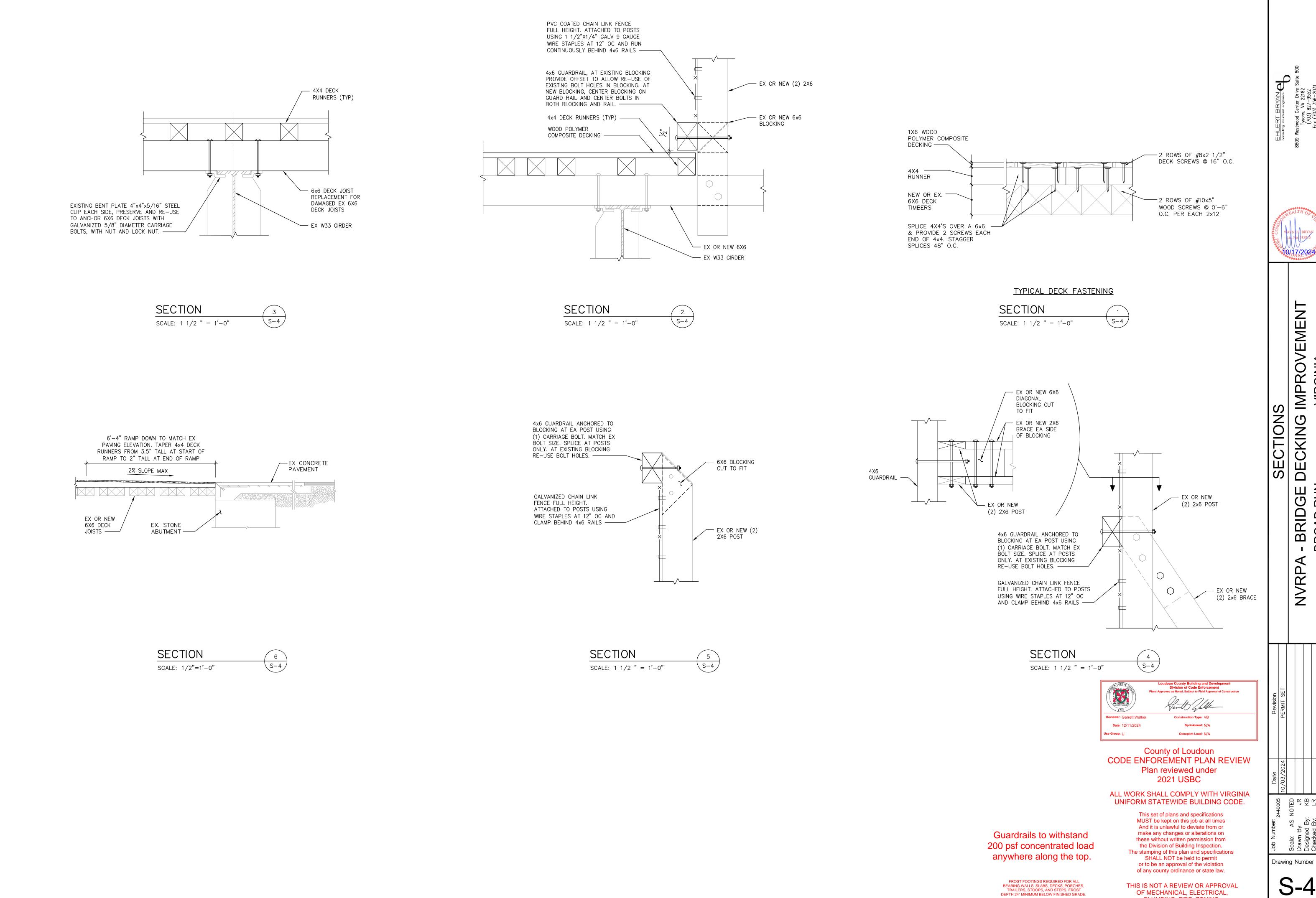
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THIS IS NOT A REVIEW OR APPROVAL OF MECHANICAL, ELECTRICAL, PLUMBING, FIRE, ZONING OR SITE PLANS.